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### 1. SURVEY AND BRIDGE SITE SELECTION

Survey and Bridge Site Assessment is the basis for planning and design and forms the main resource for bridge construction. The main objective of the Survey and Bridge Site Assessment is to identify the proper bridge site by considering socio-economic as well as technical points of view. Survey and Bridge Site Assessment is done in the following two stages:

#### Social Feasibility Survey and Technical Survey

#### 1.1 SOCIAL FEASIBILITY SURVEY

A Social Feasibility Survey is necessary to justify the construction of a requested bridge. For ranking and prioritizing the vast number of requests, the following socio-economic indicators are of utmost importance:

- Level of local participation - Size of area of influence
- Size of traffic flow

- Socio-economic benefits produced by the proposed bridge

One of the major indicators reflecting the real need of the bridge is the degree of participation by the local community or beneficiaries in the construction of the requested bridge. These indicators are assessed and measured from different points of view depending on the need and purpose of the bridge. A detailed Social Feasibility Survey is not included in this Technical Handbook, for further details refer to Social Organization Support (SOS) Manual.

#### Local Persons contacted:

Write down the names of local persons contacted or consulted during the survey work.

Name / Address	Occupation
1	
2	
3	
4	
5	
6	

### 1.2 TECHNICAL SURVEY

The technical survey includes:

- Bridge site selection and
- Topographic survey of the selected bridge site

### 2. PREPARATION FOR SURVEY

The following preparatory work must be completed before going to the field for survey:

- Collect maps with tentative location of the bridge and any available background information.
- Collect the survey equipment.

Survey equipment consists of the following materials:

#### For Survey by Abney Level

- Abney Level, Survey Form & Checklist
- Measuring Tape (50 or 100m and 3m)
- Nylon Rope (Min. 50m)
- Masons Thread and Plumb Bob
- Red Enamel Paint and Paint Brush
- Marker Pen, Scale and A3 Graph Paper
- Camera and Film Roll
- Hammer
- Ranging rod (can be prepared at site also)
- Note Book, Calculator & Pencil

#### For Survey by Theodolite

- Theodolite
- Staff and Plumb Bob
- Measuring Tape (50m and 3m)
- Red Enamel Paint and Paint Brush
- Marker Pen, Scale and A3 Graph Paper
- Camera and Film Roll
- Hammer
- Survey Form and Checklist
- Note Book & Pencil
- Calculator

### 3. GENERAL DATA COLLECTION

General data is required for needs assessment and construction planning of the proposed bridge. Collect the following general data and information:

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### 3.1 LOCATION OF BRIDGE SITE

	Left Bank	Right Bank
VDC		
Ward No.		
Ilaka No.		
District No.		
Zone		

Draw a bridge site location map covering the proposed bridge's area of influence as shown in the example below. The map should contain the following information:

- River system with names and river flow direction
- Location of proposed bridge and traditional crossing point
- Location of the nearest bridge (approximate walking distance from the proposed bridge site)
- Existing trail system and, if required, specify length of new trail for access to the proposed bridge
- Location of the next villages, health post, school and other important places with approximate distances from the bridge site

#### **Bridge Site Location Map:**

#### 3.2 NATURE OF CROSSING AND FORDABILITY

Assess period of time the river cannot be crossed in one year?

..... whole year

..... months per year only

..... days during high flood only

What type of crossing facility is available at present?

### 3.3 TRAFFIC VOLUME

Traffic volume at the crossing is one of the key indicators in the need assessment of the bridge. Information should be collected by 2 methods. Count traffic volume at the traditional crossing point for at least <u>one day</u>. And then interview the local people to form a broader impression of the traffic volume throughout the year.

		Average number of traffic per day
C la tra ff	Porters	
Goods trainc	Pack animals	
	Persons	
Non-goods traffic	Animals	

### 3.4 WIDTH OF WALKWAY

The standard width of walkway in this handbook is 70 cm or 106 cm.

In most cases the 70 cm walkway is sufficient. In stances of heavy traffic, mule and pack animal passage carrying bulky goods, or if the crossing is on a main trail, a 106 cm walkway is necessary.

Discuss this issue with the local people, informing them that more work, especially collection of stones, is required for the 106 cm walkway.

Recommended width of walkway:

70 cm

106 cm

.....

Determine the purpose of the traffic by interviewing the persons crossing and the local people as per the table below. This will indicate the importance of the crossing.

Access to	Yes	No
School		
Health post/Hospital		
Bazaar/Market		
Post office/Telephone		
Road head		
Farming		
Others (specify)		

#### 3.5 LOCAL PARTICIPATION

Tentative preliminary required number of mandays: Mandays for skilled Labor: = 1.3 x span [m] + 400 Mandays for unskilled Labor: = 5 x Span [m] + 1300

Assess the availability of local participation for bridge building from within the concerned local community

By Whom	Type of Participation/Support
Local Community	
User's Committee	
VDC	
DDC	
Local NGO	
Individual	

#### 3.6 **TRANSPORTATION DISTANCE**

Tune of Timenenont	Name of nearest Roadhead/Airstrip etc.	Distance from site up to Rroadhead/Airstrip			
Type of Tiransport		Km/Kosh	Porter days	By Mule, days	
Served by Trucks					
Served by Tractors					
Airstrip					
Helipad					

#### 3.7 **AVAILABILITY OF LOCAL MATERIALS**

Assess the availability of local materials needed for the bridge construction.

\*Do not overestimate haulage distance. Identify the nearest collection place for stone, gravel and sand.

Description	Haulage Distance*, m	Remarks
Stones		
Natural Gravel		
Sand		
Wood		
Bamboo		

#### 3.8 AVAILABILITY OF LOCAL BRIDGE BUILDERS

Are there any local bridge builders? If yes, record their names.

Names	Skill (Mason, Bridge Fitter)	Village / Address

#### 3.9 **TEMPORARY CROSSING**

Is temporary crossing is necessary during the construction of	f the br	ridge ?	Yes	No
If yes, what kind of temporary crossing do you propose?		Ferry	Cable car	Temporary bridge, m span.

### 4. BRIDGE SITE SELECTION

The main purpose of the technical field survey is to select the appropriate bridge site. The site should optimally serve the local people. The selected site must be economically justified and have along life span:

- fulfill the general conditions (listed below) have stable bank and slope conditions
  - have Favorable river conditions
- have shortest possible span

#### 4.1 GENERAL CONDITION

The bridge site should fulfill a number of general conditions:

- Traditional crossing point Minimum free board
- Maximum bridge span Space for the bridge foundations
- $\Rightarrow$  Use the checklist on following pages to evaluate the general condition at the survey site:

Foaturos	Condition	Site Assessment		
1 eutites	Conduiton	Favorable	Unfavorable	
<ul> <li>Traditional crossing point</li> <li>The bridge site should be selected at or near to the traditional crossing point.</li> <li>Traditional Crossing Point Crossing</li></ul>	<ul> <li>Favorable: Selected site is at or nearby traditional crossing point</li> <li>Unfavorable: Selected site is far from the traditional crossing point</li> </ul>			
<ul> <li>For major river, detour up to 500 m u/s and 500 m d/s from the traditional crossing point may be acceptable.</li> <li>Bridge span At local level bridge can be built only up to 120 m span. </li> <li>Max Bridge Span 120 m 120 m Rome Rome • Measure tentative span. • Measure tentative span. </li> </ul>	Favorable:span, $\ell$ is equal or shorter than 120 mUnfavorable:span, $\ell$ is longer thanM120M120M120			

Features	Condition	Site Assessment	
F eatures	Condition	Favorable	Unfavorable
Level Difference between two Banks The level difference between the two foundation blocks should not be more than <i>l</i> /25.          Bridge Span (1)         n         n         m         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n         n	<b>Favorable</b> : <b>h</b> is equal or less than $\ell/25$ <b>Unfavorable</b> : <b>h</b> is bigger than $\ell/25$		
<ul> <li>Measure the level difference between the foundations of two banks.</li> <li>Compare with the condition.</li> <li>Space for Foundation</li> <li>Foundation should be placed at least 3 m behind the soil slope and 1.5 m behind the rock slope.</li> </ul>	Favorable:Condition can be fulfilledUnfavorable:Condition can not be fulfilled		

Faatunes	Condition	Site Assessment	
	Conduiton	Favorable	Unfavorable
Slope Profile Bridge foundation should be placed behind the line of the angle of internal friction. (Angle of internal friction is the angle of slope of soil or rock at which it is still stable and does not slide).	Favorable:       Condition can be fulfilled         Unfavorable:       Condition can not be fulfilled	RB LB	RB LB
• Draw a slope line of 35° (angle of internal friction) in case of Soil slope and 60° in case of Rock slope.			
• Foundation should be placed behind this line.			
• Check if these conditions can be fulfilled.			

Foaturos		Co	ndition	Site Ass	essment
Гешигез		Co	namon	Favorable	Unfavorable
<b>Free Board</b> Clearance between (Free board) the lowest point of the bridge and the higher not be less than 5 m. For this, sufficient clearance between lower walkway s maintained.	st flood level (HFL) should addle and HFL should be				
Bridge Span (1)		<b>Favorable</b> Clearance betwee saddle and HFL is	<b>Unfavorable</b> n lower foundation s:		
	span	not less than:	less than:		
	up to 55 m 55 - 65 m 66 - 85 m 86 - 110 m 111 - 120 m	7.5 m 8.0 m 9.0 m 10.0 m 10.5 m	7.5 m 8.0 m 9.0 m 10.0 m 10.5 m		
Identify HFL by local observation and villagers' information.					
• Calculate available clearnace and compare with the requirement.					
• <b>Exception</b> : At flat or wide river banks free board may be reduced. At gorge free board has to be increased.					

### 4.2 RIVER CONDITION

The selected bridge site must have favorable river condition. Accordingly, a bridge should be located: a) on a straight reach of the river, b) beyond the disturbing influence of larger tributaries, c) on well defined banks.

Use the following check list to evaluate the river condition:

Features	Condition		Site Ass	Site Assessment	
			Favorable	Unfavorable	
<b>River Flow</b> In order to protect the bridge from sudden over-flooding and strong erosion, the bridge site should not be located near the confluence area of two rivers.					
Right Wrong Right	Favorable:	Bridge site far from the confluence			
<b>River Bed</b> The river bed at the selected bridge site should be stable without the possibility of erosion or filling up		confluence			
with bed load (boulders, gravel, silt, or sand)					
	Favorable:	River bed is not erosive not filling up			
	Unfavorable:	River bed is erosive or filling up			
River bed filled up after heavy flood					

### 4.3 SLOPE AND BANK CONDITION

A bridge should be located at the site with safe and stable slope and bank condition. The surveyor must identify any potential instability features or failure modes of the soil or rock slope and bank.

#### If the slope and bank is soil, potential instability features and failure modes are:

- bank erosion
- toppling instability of the bank
- erosion of the slope
- land slide

#### If the slope and bank is rock, potential instability features and failure modes are:

- plain failures in a rock slide along the slope.
- wedge failure leading to the fall of rock mass.
- toppling, leading to the fall of rock blocks.
- rotational slide, similar to the landslide in a soil slope. Such failure is likely when the material of the rock is very weak (soft rock) and the rock mass is heavily jointed and broken into small pieces.

To avoid the above instability features, use the following checklist to evaluate the slope and bank of the selected site

Short Span Trail Bridge Standard



Featuras	Condition	Site Ass	sessment
	Conución	Favorable	Unfavorable
Bank Erosion         The river bank should not show sign of erosion.         Image: Comparison of the river current         Image: Comparison of the river current	Favorable: No Fresh erosion Unfavorable: Presence of Fresh erosion		
Slope Profile Select applicable profile from pictures given below. The slope profile should be smooth. $\underbrace{ \begin{array}{c} \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\$	Favorable:Slope profile is smooth to partially cut outUnfavorable:Slope profile is strongly cut out	RB LB	RB LB



Features	Condition	Site Ass	essment
1 eaures	Condition	Favorable	Unfavorable
<text></text>	Favorable:No river undercuttingUnfavorable:River undercutting is active or there is potential	RB LB	RB LB
Landslide caused by river undercutting			
Inclined Trees			
Selected site should not have inclined trees which indicate an active land slide.	Favorable: Inclined trees not present Unfavorable: Inclined trees present	RB LB	RB LB

Features	Condition		Site Assessment	
1'euures		onumon	Favorable	Unfavorable
Seepage or Swampy Area The bank slope should not have any seepage or swampy area which may lead to slope instability.	Favorable: Unfavorable:	Seepage or swampy area is absent Seepage or swampy area is present	RB LB	RB LB
Gully Erosion				
No signs of gully erosion should exist within the vicinity of the selected site.	Favorable: Unfavorable:	No sign of gully erosion or only light gully erosion Heavy gully erosion exists	RB LB	RB LB

Fortures	C	andition	Site Ass	essment
reatures	C		Favorable	Unfavorable
Slipped (Slump) Soil Mass         The bridge should not be located on already slipped soil masses.         Slipped (Slump) Soil Mass         Slippe (Slump) Soil Mass         Slippe with slipped soil mass         Slippe with slipped soil mass         Slippe failure model	Favorable: Unfavorable:	There <b>are no</b> back scars or signs of soil mass movement There <b>are</b> back scars or signs soil mass movement	RB LB	RB LB

Foatures	Condition St		Site Assessment	
<i>Features</i>	Conauton	Favorable	Unfavorable	
If the river bank is ROCK Plain Failure Plain failure leads to the slide of rock layers along the slope. The rock bank/slope of the selected site should not have any feature of plain failure.	<ul> <li>Favorable:</li> <li>Plain failure will not take place, if <ul> <li>Bedding/fracture plain is subparallel to the slope</li> </ul> </li> <li>Bedding/fracture plain is parallel to the slope but inclination is less than 35°</li> <li>Unfavorable: <ul> <li>Plain failure will take place, if</li> <li>Bedding/fracture plain is parallel to the slope and inclination is greater than 35°</li> </ul> </li> <li>Presence of old slided rocks</li> </ul>	RB LB LB	RB LB	

Easturas	Condition	Site Ass	essment
Fediures	Conauton	Favorable	Unfavorable
Wedge Failure         Any form of wedge failure leads to sliding of rock masses. The rock bank/slope should not have wedge failures or potential wedge failures.         Image: Comparison of the state of the s	<ul> <li>Favorable:</li> <li>Wedge failure will not take place, if <ul> <li>There are no fracture plains facing each other</li> </ul> </li> <li>There are two or more intersecting fracture plains but the inclination of its line of intersection is less than 35°</li> <li>There are two or more intersecting fracture plains but the inclination of its line of intersection is opposite to the slope</li> </ul>	RB LB RB LB	
<ul> <li>Direction of movement</li> <li>Fracture Plane</li> <li>Slip Plane</li> <li>Plane 2</li> <li>Plane 1</li> <li>Wedge</li> <li>Bistope face</li> <li>Wedge Failure Model</li> </ul>	<ul> <li>Unfavorable:</li> <li>Wedge failure will take place, if</li> <li>There are two or more intersecting fracture plains and the inclination of its intersection line is more than 35° to the slope</li> <li>Presence of old slided wedge</li> </ul>		RB LB RB
<ul> <li>Identify if there are fracture plains facing each other (intersecting)</li> <li>Check the inclination of the intersection line</li> <li>Compare with the condition</li> </ul>			LB

Features	Condition	Site Ass	essment
	Conauton	Favorable	Unfavorable
<b>Toppling Failure</b> The rock bank/slope should not have any features of toppling failure.			
	<ul> <li>Favorable:</li> <li>Toppling will not take place, if <ul> <li>the Rock slope is less than 60°</li> </ul> </li> <li>there is no formation of rock blocks</li> </ul> <li>there is formation of rock block but</li>	RB LB RB LB	
Potential toppling failure	b/h (width of the block/height of the block) is $> 1$	LB	
Topling Failure model	<ul> <li>Unfavorable:</li> <li>Toppling will take place, if</li> <li>there is formation of vertically elongated rock blocks in the steep slope &gt; 60° and the blocks are tilted towards slopes</li> <li>old toppled rock blocks are present</li> </ul>		RB LB RB LB
<ul> <li>Identify if there is formation of vertically elongated rock blocks (cubes) due to vertical and horizontal fracture planes/joints</li> <li>Estimate inclination of the slope</li> <li>Estimate inclination and orientation of the rock block and compare with the conditions</li> </ul>			

Features	Condition	Site Ass	essment
	Conumon	Favorable	Unfavorable
Translational Failure (Slide)         The rock slope should not have any potential of translational failure.         Image: Constraint of translational failure.	<ul> <li>Favorable:</li> <li>Sliding will not take place, if <ul> <li>slope is hard rock</li> </ul> </li> <li>slope is soft rock but not weathered</li> <li>slope is soft rock and weathered but not steeper than 40°</li> </ul> <li>Unfavorable:</li>	RB LB RB LB RB LB	
Translational failure (sliding) of soft rock slope	<ul> <li>Sliding will take place, if</li> <li>Slope is highly weathered soft rock and stoop &gt; 40°</li> </ul>		RB
back scar failure(slide) model	<ul> <li>Presence of back scar or old slide</li> </ul>		LB     RB     LB
<ul> <li>Identify type of rock and its weathering grade</li> <li>Estimate inclination of the slope</li> <li>compare with the conditions</li> </ul>			

#### 4.4 EVALUATION OF THE BRIDGE SITE

After completing investigation of the site as per chapter 4.1 to 4.3, categorize the bridge site as

Good		Questionable Bad
Good	:	All or most of the features are <b>favorable</b> and if the surveyor is confident about the stability of the slopes. Proceed with further survey work.
Questionable	e :	Most of the features are <b>favorable</b> and some are <b>unfavorable</b> . The site is questionable. In this case, further detailed investigation by an experienced geo-technical engineer is necessary. For detail refer to the SBD Survey Manual.
Bad	:	Most of the features are unfavorable. Reject site.

As far as possible, the bridge site should be selected at a location where protection works will not be required. If protection works are unavoidable determine the required special structures like retaining walls, drainage channels, etc. A tentative design with dimensions and location of these structures should be illustrated in a sketch showing a plan view and a typical section. But it is best to **avoid bridge sites which require river protection works**.

Write down remarks, if any and note also proposed special structures, if necessary: retaining walls, gabion walls, drainage, lining of existing irrigation canal, river protection works etc. Mention height, length and location of these structures and also reflect it in the sketch of plan (page no 34).

**Remarks:** 

### 4.5 CLASSIFICATION OF SOIL AND ROCK

Identification of Soil and Rock type is required for appropriate foundation design. Soil and Rock can be broadly classified and categorized for the foundation design proposes as per the following tables.

Soil Type			Soil Par	ameters	Unit	Applicable
		How to identify	Bearing capacity kN/ m <sup>2</sup>	Angle of internal Friction, φ°	Weight γ, kN/m <sup>3</sup>	Foundation Design
<b>Coarse Grained Soils</b> More than half of the materials are individual grains visible to the naked eye (grain size bigger than 0.06 mm)	Gravelly Soils	Estimate the percentage (%) of coarse grains larger than 6 mm If, more than half of the coarse fraction is larger than 6 mm, the soil is Gravelly Soil	400-600 ( <b>400</b> )	32-38 ( <b>35</b> )	19	achor
	Sandy Soils	If, more than half of the coarse fraction is smaller than 6 mm grain size, the soil is Sandy Soil	200-300 ( <b>200</b> )	31-37 ( <b>33</b> )	18	Ar
<b>Fine Grained Soils</b> More than half of the materials are individual grains <b>not</b> visible to the naked eye (grain size smaller than 0.06mm)	Silty Soils	Prepare moist soil ball from the soil sample and cut it with a knife. If, the cut surface is dull or scratched, the soil is Silty Soil	150-200 ( <b>150</b> )	30-32 ( <b>30</b> )	17	adman
	Clay	Prepare moist soil ball from the soil sample and cut it with a knife. If, the cut surface is smooth and shiny, the soil is Clay.	100-200 ( <b>100</b> )	9-25 ( <b>22</b> )	16	De

#### Ratio of coarse grains



For estimation of percentage (%) of coarse grain use the figure.

### **Rock Classification**

			Degree of		Rock Parameters		
Rock Type	Examples	How to identify	Fractures or Weathering	How to identify	Bearing Capacity, kN/m <sup>2</sup>	Angle of Sliding Friction, φ°	Applicable Foundation Design
ard ock	Quartzite Limestone	Gives metallic sound after hammer blow	Rock is sound and fresh to fairly weathered	Rock has no sign of weathering or only faint signs of weathering up to 1-5 cm thickness	1500-2000 ( <b>1500</b> )	35-50 ( <b>40</b> )	Drum Anchor in Hard Rock
H R	Dolomite etc.		Highly fractured rock and fresh to fairly weathered	In the rock exists widely open cracks, fractures and bedding	1500	35-50 ( <b>40</b> )	Drum Anchor in Fractured Rock
ock	Phylite Slate	Gives dull sound after hammer blow	Fresh	No sign of weathering	1300	25-40 ( <b>30</b> )	Drum Anchor in Fractured Rock
Soft Ro	Siltstone Claystone Schist etc.		Fairly to highly weathered	<ul> <li>Most of the original rock has been seriously altered</li> <li>Rock can be broken by hand</li> </ul>	600-750 ( <b>650</b> )	25-40 ( <b>30</b> )	Deadman Anchor

#### 4.6 IDENTIFICATION OF SOIL AND ROCK

Excavate a test pit with a depth of up to the estimated foundation level (but not less than 2.0m) or up to the bed rock at the proposed foundation locations. If the bank/slope is soil, investigate each layer of soil in the pit and classify according to the Soil Classification chart, filling in the soil investigation table as per the following example.

Right Bank (at location	of Main Anchorag	e Block)				SOIL	
		Soil Type	1	Soil Parameter			
Sketch	Depth from Surface, m		Bearing Capacity, kN/m <sup>2</sup>	Angle of Internal Friction, φ	Unit Weight γ, kN/m <sup>3</sup>	Remarks	

Left Bank (at location of Main Anchorage Block)

SOIL

			Soil Parameter				
Sketch	Depth from Surface, m	Soil Type	Bearing Capacity.	Angle of Internal	Unit Weight	Remarks	
	Surrace, III		kN/m <sup>2</sup>	Friction, φ	$\gamma$ , kN/m <sup>3</sup>		

Right Bank (at location of windguy block of up stream)

	Depth from Surface, m	Soil Type	Soil Parameter				
Sketch			Bearing Capacity, kN/m <sup>2</sup>	Angle of Internal Friction, φ	Unit Weight γ, kN/m <sup>3</sup>	Remarks	

#### Left Bank (at location of windguy block of up stream)

Sketch	Depth from	Soil Type	Bearing	Angle of	Unit	Remarks
Sketen	Surface, m	Son Type	Capacity,	Internal	Weight	i i i i i i i i i i i i i i i i i i i
			kN/m <sup>2</sup>	Friction, φ	γ, kN/m <sup>3</sup>	

Standard		
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SOIL

SOIL

		C - 1 T				
Skotah	Depth from		Bearing	Angle of	Unit	Domontza
Sketch	Surface, m	Son Type	Capacity,	Internal	Weight	Remarks
			kN/m <sup>2</sup>	Friction, φ	γ, kN/m <sup>3</sup>	

### Left Bank (at location of windguy block of down stream)

	Depth from Surface, m	Soil Type	Soil Parameter				
Sketch			Bearing Capacity, kN/m <sup>2</sup>	Angle of Internal Friction, φ	Unit Weight γ, kN/m <sup>3</sup>	Remarks	

### SOIL

SOIL

Survey Form and Checklist

(					
		Rock Pa	rameter		
Rock Type	Degree of	Bearing	Angle of	Applicable Foundation Design	
	Fracture/Weathering	Capacity,	Sliding	Applicable Foundation Design	
		kN/m <sup>2</sup>	Friction, φ°		

**Left Bank** (at location of Main Anchorage Block)

	U	kN/m <sup>2</sup>	Friction, φ°	

**Rock Parameter** 

Angle of

Sliding

**Applicable Foundation Design** 

If the bank/slope is rock, investigate the rock type according to the Rock Classification chart, filling in the following rock investigation table.

Bearing

Capacity,

**Degree** of

Fracture/Weathering

### Right Bank (at location of Main Anchorage Block)

**Rock Type** 

#### ROCK

Remarks

ROCK

Remarks

Right Bank (at location of windguy block of up stream)

		Rock Pa	arameter		
Rock Type	Degree of	Bearing	Angle of	Applicable Foundation Design	Remarks
Rock Type	Fracture/Weathering	Capacity,	Sliding	Applicable Foundation Design	ixemarks
		kN/m <sup>2</sup>	Friction, φ°		

#### **Left Bank** (at location of windguy block of up stream)

### Right Bank (at location of windguy block of down stream)

-	0				
Γ			í I		
l			1		
l			1		
l			1		
l			1		
l			1		
l			1		
l			1		
l			1		
l			1		
l			1		
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I			1		
L			1 1		

eft Bank (at location of windguy block of down stream)												

### Volume II, Form No. 1

ROCK

ROCK

ROCK

### 5. TOPOGRAPHIC SURVEY

After final selection of the bridge site, the surveyor proceeds with the topographic survey. The purpose is to:

- provide a topographic map of the bridge site with details relevant to the bridge design
- establish axis pegs and bench marks for use during construction of the bridge

#### 5.1 SURVEY AREA

Area to be covered by the topographic survey:,

- A profile along the bridge axis covering up to 25m behind the main anchorage blocks, for bridges without windguy arrangement.
- A profile along the bridge axis covering up to 25m behind the main anchorage blocks **and** a topographic plan covering the area of 10m upstream and 10m downstream from the tentative location of the windguy foundations; for bridges **with windguy** arrangement

### 5.2 SETTING OF BRIDGE CENTERLINE

Fix the bridge centerline with two permanent axis points  $\mathbf{L}$  on the left bank and  $\mathbf{R}$  on the right bank. Permanent axis points  $\mathbf{L}$  and  $\mathbf{R}$  should be fixed on rock out crop along the bridge centerline, if available. If rock out crop is not available, these points should be marked on a boulder sufficiently embedded into the ground as per the sketch below:



Additional survey points along the centerline should be fixed to survey the bridge axis profile as shown in the sketch below. These survey points should be fixed at breaking points of slope and terraces, which will accurately indicate the topography of the bridge axis. The profile should cover 25m behind the main anchorage block up to the edge of the river flow.



#### 5.3 SURVEY METHODS

There are two options for conducting the topographic survey. Depending upon the span and type of bridge, a profile along the bridge axis or a more detailed survey including contour lines will be necessary.

• Profile along the Bridge axis:

Generally bridges up to 120 m do not need windguy arrangement. A detailed profile along the selected bridge axis is sufficient for this bridge design. A topographic profile can be taken by the Abney level, however for fixing precise levels a Level Instrument is necessary.

• For bridges over 120 m span, a Theodolite should be used to carry out the topographic survey.

### 5.4 SURVEY BY ABNEY LEVEL

The main function of the Abney Level is to measure the **vertical angle**  $\varphi$ . By measuring the slope distance **d** between the survey points with a measuring tape, the horizontal distance **D** and the vertical difference of elevation  $\Delta H$  can be calculated.

The principle of measuring the vertical angle by the Abney Level is illustrated in the sketch and procedure described below:



To take a profile along the bridge axis, the surveyor should first set the exact centerline as described in chapter 5.2. There are two methods of setting the centerline.

#### By measurement tape or nylon rope and plumb bob:

This method is accurate enough for span up to 50m. The survey points along the bridge centerline are fixed with the help of a measurement tape or nylon rope and a plumb bob as per procedure shown in the sketch below.



#### By Bamboo or Wooden Sticks or Ranging Rods:

This method is applied for span above 50m. In this method the survey points along the bridge centerline are fixed with the help of Bamboo or Wooden Sticks or Ranging Rods. Fix a Stick at each axis point  $\mathbf{L}$  and  $\mathbf{R}$  in **vertical** position. Now the surveyor can aim at other points along the bridge centerline line of  $\mathbf{L}$  and  $\mathbf{R}$ . By fixing in line additional survey points behind and in front of  $\mathbf{L}$  and  $\mathbf{R}$ , more points can be gained along the bridge centerline ranging as per the procedures shown in the sketch below:



Proceed with the survey of bridge profile after having fixed the centerline. Measure the vertical angles and sloped distances between the points of the centerline. The measured vertical angle between axis points L-R and R-L should be checked for error correction.

The "error-factor" is calculated with the following formula:

$$\mathbf{E} = \frac{(\pm \varphi_{LR}) + (\pm \varphi_{RL})}{2} = \pm \mathbf{E} \qquad ; \qquad \text{Corrected angle } \varphi' = \varphi \pm \mathbf{E}$$

Compute the horizontal distances and elevations of the corresponding survey points with the corrected vertical angles as per trigonometry.

Enter the measurements and calculations into the **Bridge Axis Profile by Abney Level** sheet.

Draw a plan view with the bridge axis (centerline), axis points  $\mathbf{L}$  and  $\mathbf{R}$ , with all the benchmarks and fixed objects like trees, houses etc. Give distances and directions from the reference points so that the axis points and benchmarks can be located during the construction. A plan view is necessary only when a windguy arrangement needs to be considered in bridge design.

**Plan** (indicate river flow and north direction)

Draw a sketch of the profile/cross section of the bridge axis (centerline) with axis points  $\mathbf{L}$  and  $\mathbf{R}$ , with all the survey points and topographic features, including tentative position of the bridge foundations, low water level and high flood level.

**Profile/Cross Section** 

# **BRIDGE AXIS PROFILE BY ABNEY LEVEL**

Page No. 1

Bridge Name		Distric	t:		Surveyed by :			Date :			
		Horizontal Die A Vertical a Menurest stoppe da Ce should equal to Aa	stance, D B B C C C C C C C C C C C C C C C C C	A po d		$D = d x \cos \varphi'$ $\Delta H = d x \sin \varphi'$ $H = R.L. of Station \pm \Delta H$					
STATION	POINTS	SLOPE DISTANCE d m	VERTICAL ANGLE φ	Corrected VERTICAL ANGLE \overline{\overline{\phi}}	HORIZONTAL DISTANCE D m	VER DIST ±	FICAL ANCE AH	REDUCED LEVEL (ELEVATION) H m	REMARKS (Description of Points)		

Remarks:

1. Applicable only for suspended type bridge up to 100m span and suspension type up to 70m.

2. For other cases use Theodolite.

# **BRIDGE AXIS PROFILE BY ABNEY LEVEL**

Volume II, Form No. 1

Bridge Name :		District	•	S	Surveyed by :		Date :		
STATION POINTS		SLOPE DISTANCE d m	VERTICAL ANGLE φ	Corrected VERTICAL ANGLE \	HORIZONTAL DISTANCE D m	VER' DIST ±	TICAL YANCE AH	REDUCED LEVEL (ELEVATION) H m	REMARKS (Description of Points)

Page No. 2

#### SURVEY BY THEODOLITE

When the span of the bridge is more than 120 m or when a windguy arrangement needs to be included in the bridge design, the survey is conducted with a Theodolite.

For proper use of a Theodolite, refer to the respective instruction manual, that comes with the Theodolite, and to the SBD Survey Manual.

#### **Profile Along Bridge Axis:**

Fix the bridge centerline as described in chapter 5.2. Measure distance between the axis points  $\mathbf{L}$  and  $\mathbf{R}$  by horizontal triangulation method. Triangulation is done by measuring all three angles of a triangle and length of one side as illustrated in the sketch below. For accuracy double triangulation is necessary.



Insert all the readings in the Triangulation Survey Form and calculate distance between axis points L and R by trigonometry formulas given therein.

#### **Elevation of Axis Points and Benchmarks:**

It is necessary to establish the elevations of Axis Points L and R and Benchmarks. Select elevation of one of the Bench mark  $BM_1$  as 100.00m. Establish elevations of axis points and other benchmarks by vertical triangulation as illustrated in the sketch below.



Insert the readings in the Survey form of **Summary of Triangulation and Elevations of Pegs and Benchmarks** and calculate the Elevations (reduced levels) of the points by trigonometry formulas given therein.

# TRIANGULATION



If  $\delta > \pm 0.02^{g}$  or  $0.018^{o}$  repeat the angle readings

If  $\delta > \, \underline{+} \, \, 0.02^{g} \, \text{or} \, 0.018^{o}$  repeat the angle readings

# SUMMARY of TRIANGULATION and ELEVATIONS of PEGS and BENCHMARKS

Bridge Name :	District :							Sur	vey	ed by	/:				Da	ate :			
1. Summary of Triangulation	2	. Ele	eva	tio	n						A	D	ΔH	$\Delta \mathbf{H} = \mathbf{V} \mathbf{-} \mathbf{Z} + \mathbf{I}$					
		V	- <b>I</b>	) x	tan	φ =	$=\frac{1}{ta}$	D mβ		л дн	B	D	V Z	<b>ΔH</b> =	= <b>V</b> +	Z - I			
$1^{st}$ Triangulation $D_1 = $	Г										VE	RTICA	L ANGLE			D	IFFERI ELEV#	ENCE I	N
$2^{nd} \text{ Triangulation } D_2 = \_$ Difference $\Delta D = \_$ Mean Distance $D = \frac{D_1 + D_2}{D_2} = \_$	INSTRUMENT	STATION STAFF	STATION	INSTRUMEN	HEIGHT I	MIDDLE	HAIR Z	HORIZONTAI DISTANCE	ב	F	FACE F	RIGHT	M E A N (Left + Right)	VERTICAL DISTANCE	V	Δ	Н	M E	A N
2				m	cm	m	cm	m	cr	m 1	FACE	LEFT	2	m	cm	m	cm	m	cm
$\Delta D/D = $ If $\Delta D/D > 0.0025$ repeat the triangulation	-	A	В												-				
		В	A													1	:		
		A B	MI												-	1			
REDUCED LEVELS: BMI       = 100.00         BMII       =         A       =	B	SMI	A												8		-		
B =		B B	MII												-	1			
	В	MI	в												-		-	•	-

#### **Topographic Detail Survey:**

Topographic detail survey is necessary to represent the topography of the bridge site by means of the contour lines. This is done by tacheometric survey. Tacheometric survey is done by Theodolite with stadia hairs and leveling staff. The horizontal, vertical distances and position of the points are measured by horizontal and vertical angles and stadia hair readings as illustrated in the sketches below.



• Points to be measured

Insert all the readings in the Tacheometry Survey Form and calculate the horizontal distances and elevations by the formulas given therein.

Check the stadia hair of the Thedolite before the survey. For this measure a distance of about 40m by stadia readings and actual measurement by a tape. If the difference between stadia measurement and tape measurement is more than 0.2%, corrections on calculation of horizontal and vertical distance should be applied. The distances should be corrected for error  $\Delta$  as per following Formula.

$$\mathbf{D} = (100 \text{xl} + \Delta) \text{ x } \text{Cos}^2 \phi \qquad \qquad \mathbf{V} = (50 \text{xl} + \Delta) \text{ x } \text{Sin} 2 \phi$$

Bridge Name :						District :			Su	rveyed by :			Date :		
$\ell = \ell_1 - \ell_2$ $\mathbf{D} = 100 \text{ x } \ell \text{ x Co}$	$s^2 \phi = 100$	x ℓ x Sir	n²β	$V = 50 \times \ell \times \sin 2 \varphi = 50 \times \ell \times \sin 2 \beta$ or V = D x tan $\varphi = \frac{D}{1 + 1 + 2}$							D	V Z ΔH	$\Delta$ H = V- Z +	·I	
				H = R.L. of Station $\pm \Delta$ H ΔH $\Delta$ H = V + Z - I										- I	
STATION		HORIZ	ONTAL	VERT	TICAL	TOP	MIDDLE	BOTTOM	STAFF	HORIZONT.	VERTICAL	DIFF. IN REDUCED		REMARKS	
HEIGHT OF	STAFF	CIR	CLE	CIR	CLE	HAIR	HAIR	HAIR	INTERCEPT	DISTANCE	DISTANCE	ELEVATION	LEVEL	(Description of	
INSTRUMENT, I	STATION		α		β	$\ell_1$	Z	$\ell_2$	l	D	+/ <b>-</b> V	+/- ΔH	Н	Staff Station)	
m		g/°	c / '	g/°	c / ′	m	m	m	m	m	m	m	m		
										•••					
		11	1 1												
		1 1													
			: :												

Bridge Name :			District :			Su	rveyed by :		Date :					
STATION HEIGHT OF INSTRUMENT, I	STAFF STATION	HORIZ CIR CIR	ONTAL CLE x c/'	VERT CIR	ICAL CLE 3 c/'	TOP HAIR $\ell_1$ m	MIDDLE HAIR Z m	BOTTOM HAIR $\ell_2$ m	STAFF INTERCEPT L	HORIZONT. DISTANCE D m	VERTICAL DISTANCE +/- V	DIFF. IN ELEVATION +/- ΔH m	REDUCED LEVEL H	REMARKS (Description of Staff Station)
m		8,	1 1	11										
				1 1										
				<u>; ;</u>										
						1 1						i i		
			1 1			1 1								
			::		. :									
				: :									_ : :   :	

### Page No. 2

Bridge Name : District :									Su	rveyed by :		Date :			
STATION HEIGHT OF INSTRUMENT, I	STAFF STATION	HORIZ CIR	CONTAL CLE a	VERT CIR	fical cle β	TOP HAIR ℓ <sub>1</sub>	MIDDLE HAIR Z	BOTTOM HAIR $\ell_2$	STAFF INTERCEPT ℓ	HORIZONT. DISTANCE D	VERTICAL DISTANCE +/- V	DIFF. IN ELEVATION +/- ΔH	REDUCED LEVEL H	REMARKS (Description of Staff Station)	
m		g / °	c / ′	g/°	c/'	m	m	m	m	m	m	m	m	,	
			: :	: :	!!								!!!!		
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### Page No. 4

Bridge Name	District :				Surveyed by :				Date :					
STATION HEIGHT OF INSTRUMENT I	STATION HEIGHT OF STAFF INSTRUMENT L STATION		HORIZONTAL CIRCLE α		VERTICAL CIRCLE B		R MIDDLE R HAIR Z	BOTTOM HAIR $\ell_2$	STAFF INTERCEPT l	HORIZONT. DISTANCE D	VERTICAL DISTANCE +/- V	DIFF. IN ELEVATION +/- A H	REDUCED LEVEL H	REMARKS (Description of Staff Station)
m	Similar	g / °	c/'	g/°	c/'	m	m	m	m	m	m	m	m	Suit Suiton,
												++++		
							+++					++++		
							++++	++++				++++		
												++++		
		1 1 1	111	1 1 1	1 1 1		1111	1111				$\mathbf{I}$		

### 6. PHOTOGRAPHS

Photographs of the bridge site to support its technical feasibility / topography and facilitate the bridge design. Take following photographs:

- Overall view of the bridge site from upstream indicating approximate location of bridge foundations and axis line
- Overall view of the bridge site from down stream indicating approximate location of bridge foundations axis line
- View of the right bank from left bank with approximate location of bridge foundations
- View of the left bank from right bank with approximate location of bridge foundations
- Over all top view (if possible)
- Close up view of axis points and bench marks
- Soil test pits at the location of bridge foundation blocks
- Other relevant photos

Take above photographs from the positions as per sketch below. If one picture does not cover the necessary area, take several pictures from the same spot with sufficient overlapping.



Present all the photographs on the following blank pages.

1. Overall view of the bridge site from upstream (with approximate location of bridge foundations on both R/B & L/B and axis line)

2. Overall view of the bridge site from downstream (with approximate location of bridge foundations on both R/B & L/B and axis line)

3. Overall view of the right bank (with approximate location of bridge foundation and axis line)

4. Overall view of the left bank (with approximate location of bridge foundation and axis line)

5. View of the right bank with proposed location of bridge foundation

7. View of Axis Peg/Left Bank

6. View of the left bank with proposed location of bridge foundation

9. View of Bench Mark

10. Pit excavation at .....

11. Pit excavation at .....

13. Pit excavation at .....

14. Pit excavation at .....

15. Pit excavation at .....

16. ....

### 17. ....

18. ....

19. .... 20. .....

### 7. SURVEY REPORT

The technical survey report consists of:

- Filled in Survey Forms and Checklist
- Topographic map
  - 1. Profile along the bridge axis in scale
  - 2. Contour plan of the bridge site in scale (only, if windguy arrangement is necessary)

Note Pad